



Towards eliminating the nonlinear Kelvin wake

Jack S. Keeler¹ , Benjamin J. Binder²  and M.G. Blyth¹ 

¹School of Engineering, Mathematics and Physics, University of East Anglia, Norwich NR4 7TJ, UK

²School of Computer and Mathematical Sciences, University of Adelaide, Adelaide, Australia

Corresponding authors: Jack S. Keeler, j.keeler@uea.ac.uk; Benjamin J. Binder, benjamin.binder@adelaide.edu.au; M.G. Blyth, m.blyth@uea.ac.uk

(Received 2 October 2024; revised 27 March 2025; accepted 2 May 2025)

The nonlinear disturbance caused by either a localised pressure distribution moving at constant speed on the free surface of a liquid of finite depth or a flow over a topographic obstacle, is investigated using (i) the weakly nonlinear forced Kadomtsev–Petviashvili equation which is valid for depth-based Froude numbers near unity and (ii) the fully nonlinear free-surface Euler system. The presence of a steady v-shaped Kelvin wave pattern downstream of the forcing is established for this model equation, and the wedge angle is characterised as a function of the depth-based Froude number. Inspired by this analysis, it is shown that the wake can be eliminated via a careful choice of the forcing distribution and that, significantly, the corresponding nonlinear wave-free solution is stable so that it could potentially be seen in a physical experiment. The stability is demonstrated via the numerical solution of an initial value problem for both the model equation and the fully nonlinear Euler system in which the steady wave-free state is attained in the long-time limit.

Key words: surface gravity waves, wave-structure interactions, general fluid mechanics

1. Introduction

The study of the wake produced by either a moving body on the surface of water or for flow over a topographic obstacle is a classical problem in fluid dynamics. The observed v-shaped wake pattern is a rare example of a fluid dynamics phenomenon that is well known to everyone. Attempting to minimise or even eliminate the Kelvin wake presents an important problem since in many applications its presence leads to undesirable consequences. For example, it can cause erosion to waterways and river banks (e.g. Bishop 2004), and the wave drag it produces limits the fuel efficiency of boats and ships (Havelock 1908*a*).

The mathematical description of the three-dimensional Kelvin wake has been well known for over a century (Kelvin 1887). Typically, a moving body such as a ship is represented by a pressure disturbance that propagates over the fluid surface. The angle subtended by the characteristic v-shape bounding the linear surface disturbance is usually referred to as the Kelvin wedge angle and is herein denoted by θ_k . Another way to measure the wake angle is to measure when the amplitude of the wave peaks reach a certain percentile of the highest wave Darmon, Benzaquen & Raphaël (2014), Pethiyagoda *et al.* (2021) or by fitting a line to the peaks of the highest downstream waves (Pethiyagoda, McCue & Moroney 2014; Miao & Liu 2015; Pethiyagoda *et al.* 2021); both of these methods are used to calculate a so-called ‘apparent’ wake angle, which we emphasise differs from θ_k .

In deep water θ_k can be found using a linear simplification of the full Euler system on the assumption that the disturbance to the surface is small. Treating the external moving-pressure distribution as a point disturbance leads to Kelvin’s famous result that, in a fluid of infinite depth, $\tan^2 \theta_k = 1/8$ yielding $\theta_k \approx 19.47^\circ$, which is independent of the speed of the moving-pressure distribution (see, for example, Whitham 1974). Rabaud & Moisy (2013) worked with a Froude number based on the hull length L , and excluded waves with wavenumbers smaller than a threshold value, reflecting the fact that a ship of length L cannot excite waves of wavelength much larger than L . Later, Noblesse *et al.* (2014) allowed for interference between the bow and stern of a ship and demonstrated that the apparent wake angle can be smaller than 19.47° . In shallow water θ_k also depends on the flow speed and water depth (Havelock 1908*b*) and is characterised by a depth-based Froude number, Fr , to be defined in (2.4). Herein, unless otherwise explicitly stated, when we refer to the Froude number we mean the depth-based Froude number defined in (2.4).

Despite the ubiquity of the Kelvin wake phenomenon and the large number of research articles that have been devoted to it, it remains a topic of considerable scientific interest. Concentrating initially on linear theory, the so-called ‘wave-drag coefficient’ (Havelock 1908*a,b*; Pedersen 1988; Li & Ellingsen 2016) can be used to quantify the resistance induced by the wake of a moving-pressure distribution in finite depth. In finite depth, the wave pattern induced by mono-hulls and catamarans has been characterised by models using point sources as surface pressure distributions (Zhu *et al.* 2015, 2016) and Hogner ship theory (Zhu *et al.* 2018). There has also been work done to quantify the ‘apparent’ wake angle for an arbitrary pressure distribution in infinite depth (Miao & Liu 2015). In none of these studies did the wave pattern completely disappear for any choice of external forcing.

Theoretical progress has also been made for slow-moving boats in infinite depth assuming the Froude number is small. Somewhat surprisingly, an asymptotic solution of the two-dimensional Euler system, which utilises expansions in powers of the hull-based Froude number, produces no waves at each order of approximation. This apparent paradox can be resolved by resorting to ‘beyond-all-orders’ asymptotics, and in this context, it can be shown that there are waves present downstream that are exponentially small in the Froude number (Trinh, Chapman & Vanden-Broeck 2011; Trinh & Chapman 2014). For the two-dimensional problem, Trinh *et al.* (2011) showed that downstream waves cannot be eliminated for a single-cornered ship. However, Trinh & Chapman (2014) were not able to make such a conclusive statement for more general piecewise-linear hulls and they did not discount the possibility of wave-free solutions. Lustri & Chapman (2013) and Pethiyagoda *et al.* (2021) considered the linear three-dimensional problem for infinite-depth flow over submerged sources and moving-pressure distributions and characterised the ‘apparent’ angle in terms of the system parameters.

In contrast, the fully nonlinear problem has received far less attention in the literature. From a computational perspective, recently, Pethiyagoda *et al.* (2014, 2015) and Buttle *et al.* (2018) used a numerical boundary-integral approach coupled with Newton–Krylov

and pre-conditioning techniques to solve the fully nonlinear Euler system for inviscid, irrotational flow (hereinafter referred to as the Euler system) in order to capture efficiently the steady wave patterns behind a moving object, and provided important insight into the nonlinear wave drag. Significantly, in infinite depth, Pethiyagoda *et al.* (2014, 2015) showed that as the flow speed and forcing pressure amplitude are increased, nonlinearity, via the amplitude of the external forcing, alters the ‘apparent’ wake angle and therefore the physical extent of the wavy region. More recently, nonlinear wake patterns have been computed in finite depth by Buttle *et al.* (2018), where it was demonstrated that, whilst the linear downstream wave patterns are identical for the moving-pressure problem and the topographic-obstacle problem, the nonlinear wave patterns are different.

The theoretical literature on the fully nonlinear problem is sparse. However, recently, as a first step towards examining the Euler system, Kataoka & Akylas (2023) showed how an exponential asymptotics approach may be used for smooth boat hulls in the context of a model equation. Examining smooth hulls is important as they can be used to model the bulbous hull shapes that are known to minimise wave drag (see, for example, Grosenbaugh & Yeung 1989). However, modelling smooth bodies moving at arbitrary speeds using the Euler system remains an open challenge.

In this paper, we work towards the goal of eliminating the Kelvin wake behind a moving body in a real flow by studying (i) the simplified model Kadomtsev–Petviashvili equation with a localised forcing term, originally derived by Katsis & Akylas (1987), and it is hereinafter referred to as fKP and (ii) the fully nonlinear Euler system with either a moving-pressure distribution on the surface or a topographic obstacle on the bottom. In the fKP model, we demonstrate that a generic forcing produces a trailing wave pattern similar to that found for the Euler system with a v-shaped wake whose angle we characterise as a function of the Froude number. This includes the critical case $Fr = 1$ covered by Katsis & Akylas (1987), which showed that the wake fills the entire region behind the disturbance for this flow speed.

Furthermore, we use a simple mathematical argument to show that, for the fKP system, it is possible to eliminate the wake. By a judicious choice of forcing function, we construct exact solutions for the fKP model and numerical solutions for the fully nonlinear Euler system for which the disturbance to the water surface is localised around the forcing so that the surface is undisturbed in all directions into the far field. We refer to such solutions as wave-free solutions. Strikingly, we provide evidence that the wave-free solutions are stable, meaning that they are reached in the long-time limit of an initial-value problem (IVP), leading to the possibility of observing them in a physical experiment.

We proceed as follows. In § 2 we describe the problem statement including the fully nonlinear Euler system and the fKP equation and boundary conditions. Then in § 3, we characterise the v-shaped Kelvin wake pattern that emerges for a generic choice of forcing function in the fKP model. In § 4 we discuss how to construct a wave-free steady state for the fKP model and the fully nonlinear Euler system in the presence of a suitably chosen localised forcing. In § 5 we formulate and solve numerically an IVP for the fKP model and fully nonlinear Euler system, to show that the wave-free states are stable. Finally, in § 6 we present our findings and highlight future directions for study.

2. Problem formulation

Before discussing our results we first describe the governing equations; first, the fully nonlinear Euler system and then the fKP model equation. We also illustrate how the dispersion relation for the model fKP agrees with the fully nonlinear Euler system.

2.1. The fully nonlinear Euler system

The fully nonlinear free-surface Euler equations in finite depth are derived from the Navier–Stokes equations under the assumptions of incompressibility, irrotationality and zero viscosity. In [figure 1](#) we show the physical (dimensional) domain and the labelling of the domain and boundaries. To arrive at our non-dimensional system we scale all lengths by the depth of the fluid, H , all velocities by the speed of the incoming uniform flow, U , and time is scaled by H/U . If we define $\mathbf{x} = [x, y]^T$, then the fully nonlinear set of equations for the non-dimensionalised velocity potential $\phi(x, y, z)$ and free-surface $\mathbf{r} = [x, y, z_f(\mathbf{x})]^T$ are therefore

$$\nabla^2 \phi = 0, \quad \mathbf{x} \in \mathbb{R}^2, \quad 0 < z < z_f(\mathbf{x}), \quad (2.1)$$

$$\mathbf{r}_t \cdot \mathbf{n}_1 = \nabla \phi \cdot \mathbf{n}_1, \quad \text{on } z = z_f(\mathbf{x}), \quad (2.2)$$

$$\phi_t + \frac{1}{2} |\nabla \phi|^2 + \frac{1}{Fr^2} z_f + \sigma_{press}(\mathbf{x}) = \frac{1}{2} + \frac{1}{Fr^2}, \quad \text{on } z = z_f(\mathbf{x}), \quad (2.3)$$

where $\sigma_{press}(\mathbf{x})$ is an imposed pressure distribution on $z = z_f(\mathbf{x})$ and \mathbf{n}_1 is the outwards-pointing unit normal at the free surface and the depth-based Froude number is defined as

$$Fr = \frac{U}{\sqrt{gH}}, \quad (2.4)$$

with U and H are the undisturbed physical flow speed and channel height far upstream and g is the gravitational constant (see, for example, Lannes [2013](#)). To close the system we impose

$$\phi \rightarrow x, \quad z_f \rightarrow 1, \quad \text{as } x \rightarrow -\infty, \quad \text{and } y \rightarrow \pm\infty, \quad (2.5)$$

and on the bottom we impose a no-penetration condition

$$\nabla \phi \cdot \mathbf{n}_0 = 0, \quad \text{on } z = \sigma_{topog}(\mathbf{x}), \quad (2.6)$$

where $\sigma_{topog}(\mathbf{x})$ is the shape of the topographic distribution and \mathbf{n}_0 is the outwards-pointing unit normal on the bottom. Furthermore, we define the perturbation to the free surface from its undisturbed upstream height as

$$\eta_f(\mathbf{x}) = z_f(\mathbf{x}) - 1. \quad (2.7)$$

The nonlinear system described by (2.1)–(2.6) is what shall be referred to as the fully nonlinear Euler system. In what follows we shall refer to (i) the ‘moving-pressure problem’ which only considers the surface pressure and so $\sigma_{topog} = 0$ or (ii) the ‘topographic-obstacle problem’ which only considers an obstacle over an underlying uniform stream and so $\sigma_{press} = 0$. In both scenarios, we work in a fixed reference frame in which the forcing is stationary and the Bernoulli constant in (2.3) is $1/2 + 1/Fr^2$.

2.2. The weakly nonlinear model

As derived in the context of water waves the fKP is a partial differential equation (PDE) for the height of the free surface, denoted $\eta(\mathbf{x}, t)$, in terms of an external forcing profile, $\sigma(\mathbf{x})$ that either represents a surface pressure distribution, σ_p (Katsis & Akylas [1987](#)), or the shape of a topographic obstacle, σ_t (Keeler [2014](#)). It is derived from (2.1)–(2.6) under the assumptions of (i) shallow water, (ii) small forcing amplitude, (iii) weak y -dependence and (iv) $Fr \approx 1$ (see, for example, Lannes [2013](#), for mathematically rigorous statements

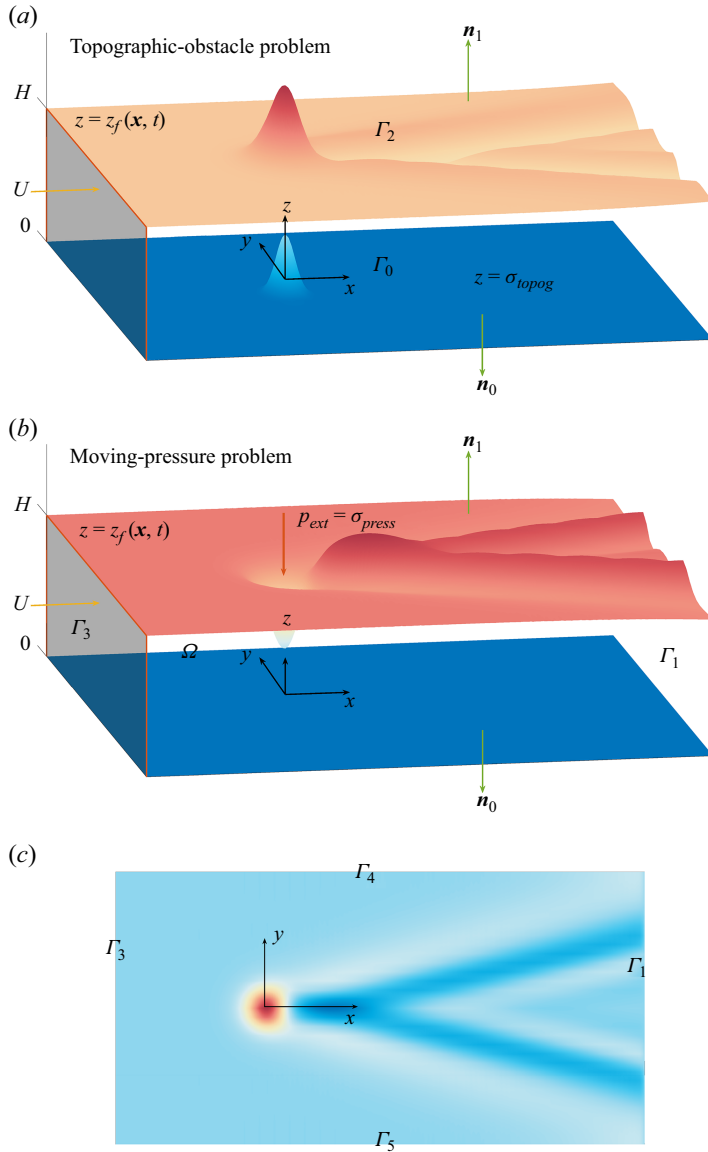


Figure 1. A sketch of the physical (dimensional) domain and labelling of the domain and boundaries. The (a) panel shows the topographic-obstacle problem, the (b) panel shows the moving-pressure problem and the (c) panel shows a plan view of the free surface to demonstrate the boundary labelling. Here, Ω is the fluid domain, Γ_0 is the bottom topography, Γ_2 is the free surface, Γ_1/Γ_3 are the downstream/upstream faces of the boundary respectively and Γ_4 and Γ_5 are the two lateral faces of the boundary.

on the validity of this model). Therefore, in contrast to the small- Fr literature stated in the introduction, our focus is on moderate and fast flow speeds. The equation is

$$\left(\eta_t + (Fr - 1)\eta_x - \frac{3}{2}\eta\eta_x - \frac{1}{6}\eta_{xxx} - \frac{1}{2}\sigma_x \right)_x - \frac{1}{2}\eta_{yy} = 0, \quad x, y \in \mathbb{R}, \quad (2.8)$$

where the subscripts indicate partial derivatives. Far upstream of the forcing, we impose the boundary conditions

$$\eta, \eta_x, \eta_{xx}, \eta_{xxx} \rightarrow 0, \quad \text{as } x \rightarrow -\infty, \quad (2.9)$$

$$\eta_y \rightarrow 0, \quad \text{as } y \rightarrow \pm\infty, \quad (2.10)$$

in accordance with (2.5). We note that (2.8) is valid in the context of no/weak surface tension, and is a modification of the Kadomtsev–Petviashvili II equation (the KPI equation has the opposite sign on the fourth derivative and is for strong surface tension).

It is worth making the link between the fKP model and the fully nonlinear Euler system explicit by examining the linear far-field wave behaviour in the absence of forcing. Substituting $\phi = x + \varepsilon \exp(i(\omega t - \mathbf{k} \cdot \mathbf{x}))$ and $z_f = 1 + \varepsilon \exp(i(\omega t - \mathbf{k} \cdot \mathbf{x}))$, with $\varepsilon \ll 1$ and $\mathbf{k} = [k_x, k_y]^T$ in (2.1)–(2.6) and truncating the system at $O(\varepsilon)$ gives the linear dispersion relation of the Euler system in finite depth with an underlying uniform flow. We can also derive a linear dispersion relation for the fKP model (2.8) by assuming $\sigma = 0$ and neglecting nonlinear terms. Written together, these are

$$\omega_{Euler}(\mathbf{k}) = k_x - \frac{1}{Fr} (|\mathbf{k}| \tanh |\mathbf{k}|)^{1/2}, \quad (2.11)$$

$$\omega_{fKP}(\mathbf{k}) = (Fr - 1)k_x - \frac{1}{2} \frac{k_y^2}{k_x} + \frac{1}{6} k_x^3. \quad (2.12)$$

By insisting that (i) Fr is close to unity and the waves are (ii) long wavelength, (iii) act on a long time scale and (iv) extend weakly in the transverse direction, it can be deduced that on writing, for constant μ ,

$$k_x \mapsto \mu k_x, \quad k_y \mapsto \mu^2 k_y, \quad \omega \mapsto \mu^3 \omega, \quad Fr \mapsto 1 + \mu^2 \lambda, \quad (2.13)$$

and assuming that $\mu \ll 1$, then ω_{Euler} becomes

$$\omega_{Euler}(\mathbf{k}) = \lambda k_x - \frac{1}{2} \frac{k_y^2}{k_x} + \frac{1}{6} k_x^3 + O(\mu^2). \quad (2.14)$$

Evidently, at leading order, (2.14) precisely coincides with the dispersion relation of the fKP in (2.12) once the scalings in (2.13) have been reversed.

In the analysis that follows, we will construct a mixture of exact nonlinear steady solutions to (2.8) and numerical steady/unsteady solutions to (2.8) and (2.1)–(2.6). The numerical solutions in both the fKP system and Euler system are found using a finite-element discretisation of the weak form of these systems, as described in [Appendix A1](#). The discretisation of the weak form of the Euler system is discussed in considerable detail in Keeler & Blyth (2024) for the two-dimensional problem; indeed, an attractive feature of this novel numerical framework is that the extension of the problem from two to three spatial dimensions is as trivial as changing a ‘2’ to a ‘3’ in a significant majority of the code.

Finally, it is worth re-emphasising that, in the calculations that follow, we label nonlinear solutions to the fKP equation by $\eta(\mathbf{x}, t)$ and the perturbed free surface in the fully nonlinear system by $\eta_f(\mathbf{x}, t)$, see (2.7). We will also use a subscript ℓ to denote a linear steady solution in either system.

3. Preliminary analysis: weakly nonlinear steady Kelvin wake patterns

Before we demonstrate how we construct wave-free solutions, it is instructive to characterise the v-shaped Kelvin wake pattern using the weakly nonlinear fKP model, and

in particular the wedge angle, for a generically chosen external forcing in terms of the flow speed, characterised by the depth-based Froude number, Fr . The procedure we follow is well known for the linearised Euler system (see, for example, Havelock 1908*a,b*; Miao & Liu 2015) and has recently been reconstructed for a different model PDE by Kataoka & Akylas (2023) that models pressure distributions in infinite depth.

We emphasise that we were unable to find any explicit analysis for the Kelvin wedge angle in (2.8) in the literature, but note the linear analysis of Katsis & Akylas (1987); restricted to critical flow, $Fr = 1$, and a time-dependent response with no explicit determination of the Kelvin wedge angle as a function of Fr . Furthermore, this analysis will reveal simple closed-form expressions for the wedge angle θ_k as a function of Fr and motivate how to eliminate the waves in subsequent sections.

To determine the Kelvin wedge angle, we examine linear steady solutions to (2.8) by assuming η is small

$$\mathcal{L}(\eta_\ell) = 3\sigma_{\ell,xx}, \quad \mathcal{L} \equiv 6(Fr - 1)\partial_{xx} - \partial_{xxxx} - 3\partial_{yy} = 0. \quad (3.1)$$

For convenience, in what follows, η_ℓ, σ_ℓ correspond to a linear solution and forcing term of (3.1), while η, σ correspond to a nonlinear solution and forcing term of (2.8). A primary advantage of using the fKP model as opposed to the fully nonlinear Euler system is that, given a linear solution η_ℓ corresponding to a forcing σ_ℓ , then a nonlinear steady solution can be written down exactly as

$$\eta(\mathbf{x}) = \eta_\ell(\mathbf{x}), \quad \sigma(\mathbf{x}) = \sigma_\ell(\mathbf{x}) - \frac{3}{2}\eta_\ell^2(\mathbf{x}). \quad (3.2)$$

We emphasise that, in the fully nonlinear Euler system, we are unable to write down a simple closed-form expression for the nonlinear solution in this fashion.

Returning to the linear problem, we solve (3.1) using the two-dimensional Fourier transform, defined as

$$\hat{\eta}_\ell(\mathbf{k}) = \mathcal{F}(\eta_\ell) \equiv \frac{1}{2\pi} \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} \eta_\ell(\mathbf{x}) \exp(-i\mathbf{k} \cdot \mathbf{x}) \, dx \, dy. \quad (3.3)$$

Introducing a polar coordinate system

$$\mathbf{x} = r[\cos \theta, \sin \theta]^T, \quad \mathbf{k} = \kappa[\cos \varphi, \sin \varphi]^T, \quad (3.4)$$

and letting $\hat{\sigma}_\ell(\mathbf{k}) = \mathcal{F}(\sigma)$, the steady linear solution to (3.1) can be written in terms of the inverse Fourier transform as

$$\eta_\ell(\mathbf{x}) = \frac{1}{2\pi} \int_0^{2\pi} \int_0^\infty \frac{3\hat{\sigma}_\ell(\mathbf{k}) \kappa \exp(i\kappa r \cos(\varphi - \theta))}{(\kappa^2 - \kappa_0^2) \cos^2 \varphi} \, d\kappa \, d\varphi, \quad \kappa_0^2 = \frac{3 \tan^2 \varphi - 6(Fr - 1)}{\cos^2 \varphi}. \quad (3.5)$$

The Kelvin wedge angle can be determined by examining the leading-order asymptotic behaviour of (3.5) far downstream, as $r \rightarrow \infty$, $|\theta| < \pi/2$. To proceed, the inner κ -integration can be achieved by extending the domain of integration into the complex plane, i.e. $\kappa \in \mathbb{C}$, and then using Cauchy's residue theorem. For subcritical and critical flow, $Fr < 1$ and $Fr = 1$, respectively, $\kappa_0^2 > 0$, and hence there are simple poles at $\kappa = \pm\kappa_0$; we only consider the pole at $\kappa = +\kappa_0$ as the integration limit in (3.5) is the positive real line. If $Fr > 1$, it is possible that $\kappa_0^2 < 0$ and this will be discussed below.

Great care has to be taken when choosing the path of integration and choice of contour indentation around the pole $\kappa = +\kappa_0$ which lies directly on the path of integration. Indeed, as remarked in Whitham (1974) for the fully nonlinear Euler system, there is an ambiguity that stems from the fact that the radiation condition states there cannot be waves upstream.

A similar problem arises in (3.5), so to remedy this, we follow Whitham (1974) and allow η_ℓ and σ_ℓ to be weakly time dependent by mapping

$$\eta_\ell(\mathbf{x}, t) \mapsto \eta_\ell(\mathbf{x}) \exp(\delta t), \quad \sigma_\ell(\mathbf{x}, t) \mapsto \sigma_\ell(\mathbf{x}) \exp(\delta t), \quad (3.6)$$

with $\delta > 0$ in (2.8), ignoring the nonlinear term. This mimics the forcing being effectively zero as $t \rightarrow -\infty$, and has the effect of altering $\mathcal{L}(\eta_\ell)$ in (3.1) to

$$\mathcal{L}(\eta_\ell) \mapsto \mathcal{L}(\eta_\ell) + 6\delta \eta_{\ell,x}, \quad (3.7)$$

and it can be shown that, now, the pole gets translated into the upper complex plane to

$$\kappa_0 \mapsto \kappa_0 + \frac{3i\delta}{\kappa_0^2 \cos^3 \varphi} + O(\delta^2). \quad (3.8)$$

The ambiguity in choosing the orientation of the indentation over the pole is now redundant. Finally, by taking the limit $\delta \rightarrow 0^+$, we recover the steady solution to (3.1).

Due to this construction, for $\cos(\varphi - \theta) > 0$, we close our path of integration in the upper half complex κ -plane by adding a quarter-circle arc from the positive real κ axis to the positive imaginary κ axis and then returning to the origin and the pole will contribute as $r \rightarrow \infty$ (we note that the contribution of the integrand on the imaginary axis is exponentially small in r). For $\cos(\varphi - \theta) < 0$ the contour gets deformed onto the negative imaginary axis and the pole does not contribute.

Taking the limit $\delta \rightarrow 0^+$, the leading-order asymptotic behaviour of the inner integral, when $\cos(\varphi - \theta) > 0$ in (3.5), is simply the residue of the integrand at κ_0 and therefore

$$\eta_\ell(\mathbf{x}) \sim \frac{3}{2}i \int_{-\pi/2+\theta}^{\pi/2} \frac{\hat{\sigma}_\ell(\mathbf{k}_0)}{\cos^2 \varphi} \exp(i|\kappa_0|r \cos(\varphi - \theta)) d\varphi, \quad \text{as } r \rightarrow \infty, \quad (3.9)$$

where $\mathbf{k}_0 = |\kappa_0|[\cos \varphi, \sin \varphi]^T$ is dependent on φ only. The limits of integration on the outer- φ integral have changed from (3.5) as the solution is symmetric about $\theta = 0$ so we only need to consider $0 < \theta < \pi$, which means that, as $\cos(\varphi - \theta) > 0$, $-\pi/2 + \theta < \varphi < \pi/2$. The dominant behaviour of the integral as $r \rightarrow \infty$ occurs when the exponent in (3.9) is stationary, i.e.

$$\frac{\partial g}{\partial \varphi} = 0, \quad \text{where } g(\varphi, \theta) = \kappa_0 \cos(\varphi - \theta) \equiv \frac{\sqrt{3}(\tan^2 \varphi - 2(Fr - 1))^{1/2} \cos(\varphi - \theta)}{\cos \varphi}. \quad (3.10)$$

The leading-order asymptotic behaviour of (3.9) as $r \rightarrow \infty$ is readily obtained using the stationary-phase formula

$$\eta_\ell(\mathbf{x}) \sim \frac{3\sqrt{\pi}(i - \lambda)}{4} \sum_n r^{-1/2} \frac{\hat{\sigma}_\ell(\kappa_0, \varphi_n)}{|g''(\varphi_n)|^{1/2} \cos^2 \varphi_n} \exp[i|\kappa_0|r \cos(\varphi_n - \theta)] + c.c., \quad (3.11)$$

where the sum is over the n permissible solutions, $\varphi = \varphi_n$, of (3.10), $\lambda = \text{sgn}(g''(\varphi_n))$ is the signum function, $g''(\varphi_n)$ is the second derivative of $g(\varphi)$ with respect to φ , $\hat{\sigma}_\ell(\kappa_0, \varphi_n) = \hat{\sigma}_\ell(\mathbf{k}_0)|_{\varphi=\varphi_n}$ and c.c. stands for complex conjugate. Physically, as can be seen from (3.11), κ_0 characterises the wavenumber of the oscillations.

When $Fr \neq 1$ the solutions satisfy the quadratic equation for $\tan \varphi$,

$$2 \sin \theta \tan^2 \varphi + \cos \theta \tan \varphi - 2(Fr - 1) \sin \theta = 0. \quad (3.12)$$

Hence, when $Fr < 1$, there are two real solutions for φ provided that

$$0 < |\theta| < \theta_k(Fr), \quad \text{where } 16(1 - Fr) \tan^2 \theta_k = 1. \quad (3.13)$$

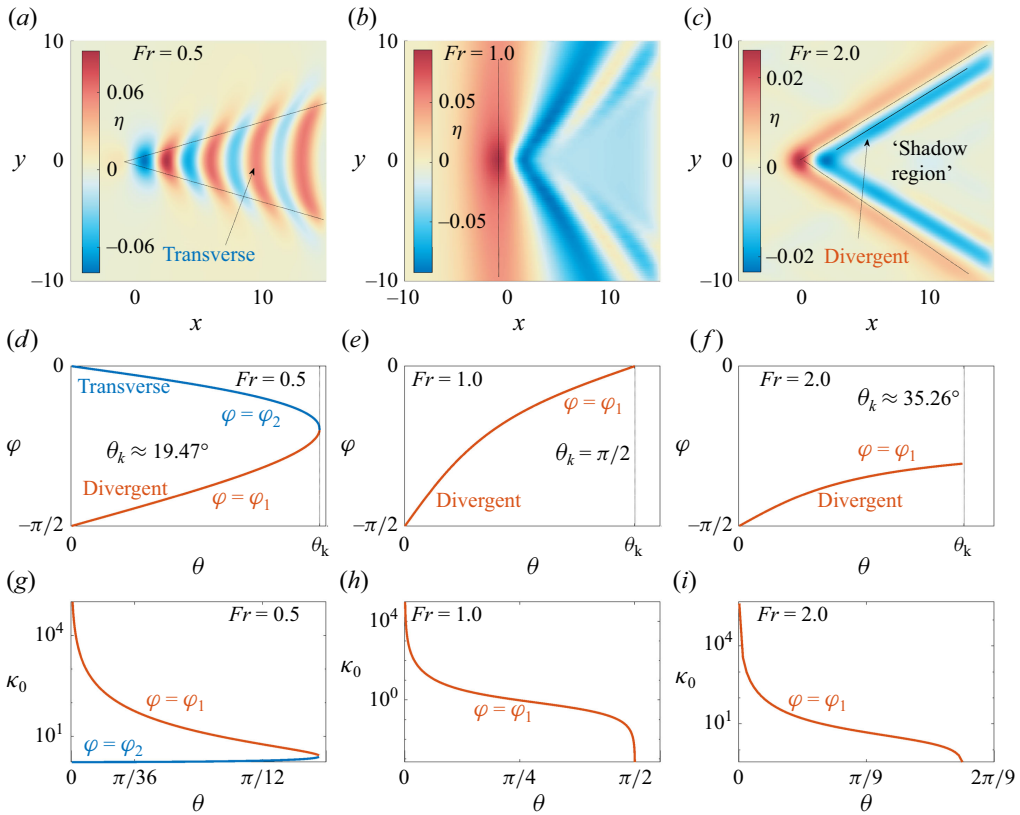


Figure 2. The left, middle and right columns show results for $Fr = 0.5, 1.0$ and 2.0 , respectively. (a)–(c) Nonlinear steady solutions of (2.8) for $\sigma(\mathbf{x}) = a \exp(-\mathbf{x} \cdot \mathbf{x})$ with $a = 0.1$. The linear Kelvin wedge angle, θ_k is indicated as dotted lines. (d)–(f) The roots of (3.10) in the range $-\pi/2 + \theta < \varphi < \pi/2$ as a function of θ for a general forcing function σ_ℓ . (g)–(i) The pole on the real axis, κ_0 , evaluated at $\varphi_{1,2}$.

The value of $\theta_k(Fr)$ is interpreted as the Kelvin wedge angle. It is indicated by the dashed vertical line in figure 2(d) for the case $Fr = 0.5$. Comparing the value of κ_0 at each root, see figure 2(g), we see that one of the solutions, φ_1 , is short wavelength and corresponds to a so-called divergent wave (that shall be discussed below) whilst the other solution, φ_2 , is long wavelength and corresponds to a so-called transverse wave that oscillates on the line $\theta = 0$, as can be seen in (3.11). For historical context, the divergent/transverse terminology was first coined in Havelock (1908b). We note the special case of $Fr = 1/2$, as shown in figure 2(d),(g), which results in $\theta_k = \arctan(1/2\sqrt{2}) \approx 19.47^\circ$, the classical Kelvin wedge angle for the linearised Euler system.

When $Fr > 1$ equation (3.12) has two solutions for any $0 < |\theta| < \pi/2$. However, for one of these solutions, φ_2 , we find that $\kappa_0^2 < 0$ for all $0 < |\theta| < \pi/2$ and so the pole in (3.5) is not on the real axis (as $\delta \rightarrow 0$) and does not contribute to the far-field wave pattern. For the other (divergent) solution, φ_1 , the transition from $\kappa_0^2 > 0$ (pole tending to the real axis) to $\kappa_0^2 < 0$ (pole tending to the imaginary axis) occurs when $\tan^2 \varphi = 2(Fr - 1)$. Inserting the latter into (3.12) we find that the permissible θ are bounded by the Kelvin wedge angle, θ_k and satisfy

$$0 < |\theta| < \theta_k(Fr), \quad \text{where} \quad 2(Fr - 1) \tan^2 \theta_k = 1. \quad (3.14)$$

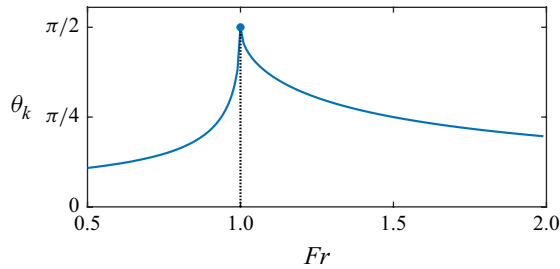


Figure 3. Kelvin wedge angle dependence on Fr using (3.13) and (3.14).

Focusing on $Fr > 1$, a key feature of the divergent wave solution is the presence of a ‘shadow region’ downstream of the forcing where there is minimal surface disturbance, see figure 2(c), and no surface disturbance at $\theta = 0$. To see this, $\varphi_1 \rightarrow -\pi/2$ and $\kappa_0 \rightarrow \infty$ as $\theta \rightarrow 0$, see figure 2(f,i). Therefore, examining (3.11), for a fixed value of r ; η_ℓ decays to zero as $\theta \rightarrow 0$, provided $\hat{\sigma}_\ell(\kappa_0, \varphi_1)$ decays sufficiently quickly as $\kappa_0 \rightarrow \infty$. The Gaussian forcing we choose in the calculations in figure 2(a–c), and discussed below, fulfils this criterion.

Finally, when $Fr = 1$ (3.10) has a unique solution, φ_1 , corresponding to a divergent wave, for any $0 < |\theta| \leq \pi/2$ which satisfies

$$\tan \varphi = -\frac{1}{2} \cot \theta. \quad (3.15)$$

Hence, the Kelvin wedge angle is $\theta_k = \pi/2$, as was first noted in Katsis & Akylas (1987).

In summary, we have determined the far-field Kelvin wedge angle as a function of Fr for subcritical and supercritical flows, as stated in (3.13) and (3.14), respectively. The wedge angle is plotted against Fr in figure 3. We note that $\theta_k \rightarrow \pi/2$ as $Fr \rightarrow 1^\pm$ and that for large Fr we have the asymptotic behaviour

$$\theta_k \sim (\sqrt{2}/2) Fr^{-1/2} \quad \text{as } Fr \rightarrow \infty. \quad (3.16)$$

This asymptotic behaviour differs from the linearised Euler system where the dependence of the θ_k on the flow speed is well known, most notably reported by Havelock (1908b), where it was found that the leading-order asymptotic behaviour for $Fr \gg 1$ is $\theta_k = \text{asin}(Fr^{-1}) \sim Fr^{-1}$ (more recently noted by Pethiyagoda *et al.* (2018)). This discrepancy can be explained by the fact that the fKP is only valid around $Fr \approx 1$.

We can compare the values for θ_k , obtained from the solution to (3.13) and (3.14), with the numerically computed nonlinear steady wave patterns, found by solving the steady form of the fKP (2.8), as shown in figure 2(a–c) for $Fr = 0.5, 1.0, 2.0$, respectively. For concreteness, we choose a Gaussian forcing $\sigma(\mathbf{x}) = a \exp(-\mathbf{x} \cdot \mathbf{x})$ and $a = 0.1$. The dashed lines in each panel indicate the Kelvin wedge angle, θ_k . This theoretically derived angle shows excellent agreement with the numerically produced nonlinear wave patterns, giving us confidence in our numerical scheme (see Appendix A1). Furthermore, the wave patterns are qualitatively the same as the fully nonlinear solutions reported in Buttle *et al.* (2018), where transverse waves are present for $Fr < 1$ but absent when $Fr \geq 1$, where instead a ‘shadow region’ appears.

Although the linear theory for the fully nonlinear Euler system is well established (see, for example, Havelock 1908b), to our knowledge this is the first time the Kelvin wedge angle and wave pattern have been calculated explicitly for a range of Fr , in the context of the fKP equation and is the first main result of this paper. Due to the simplicity of (3.1), and the fact that we have obtained simple closed-form exact equations for θ_k as a function

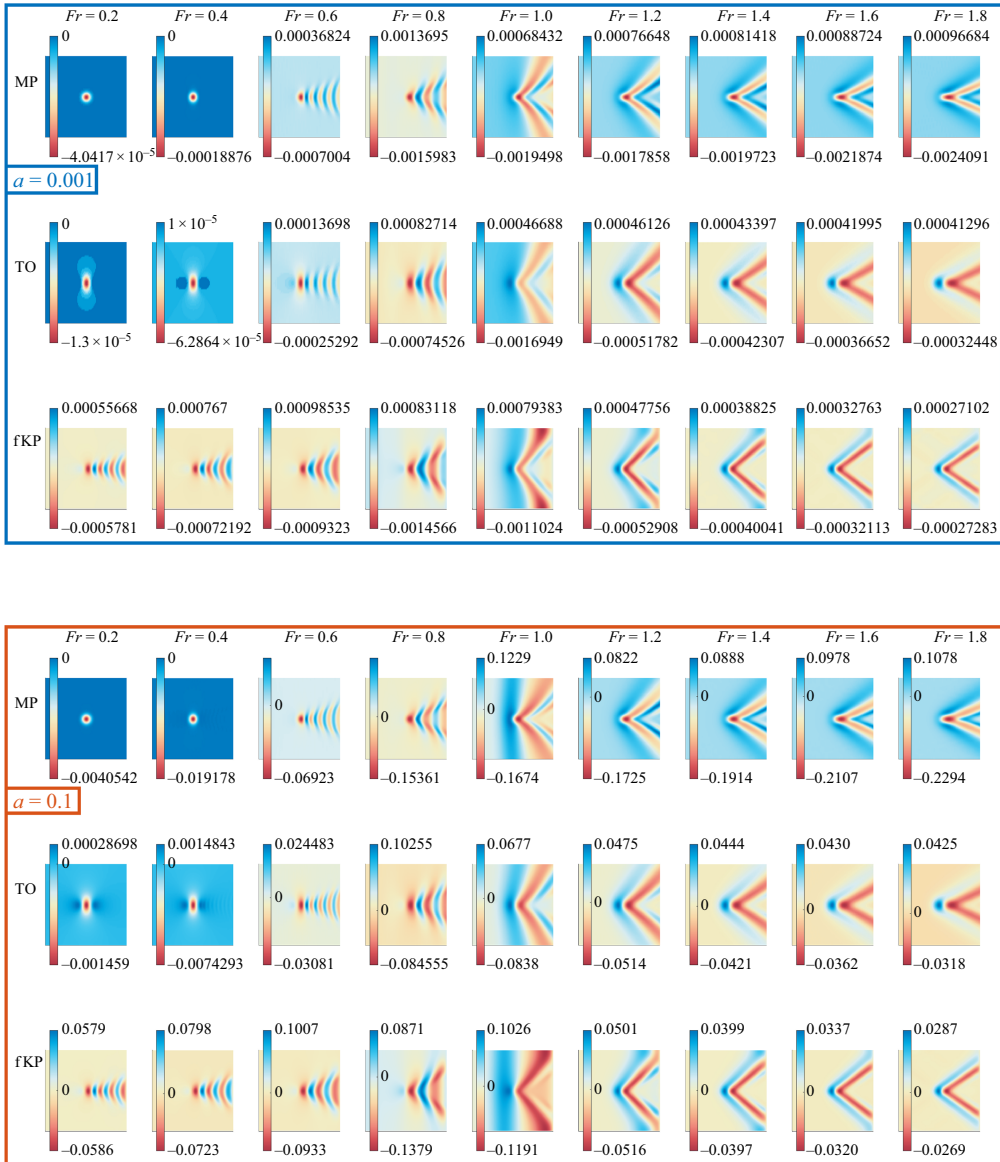


Figure 4. Comparison between nonlinear steady surface wave patterns of the fully nonlinear Euler system, $\eta_f(\mathbf{x})$ for a moving-pressure problem, (labelled MP in the top row of each box), topographic-obstacle problem (labelled TO in the middle row of each box) and the fKP model, $\eta(\mathbf{x})$ (bottom row in each box) for a range of Fr and $\sigma = a \exp(-\mathbf{x} \cdot \mathbf{x})$. Note that the colour map is different for each panel but the reference values $\min(\eta)$, 0 and $\max(\eta)$ (where applicable) have been labelled on the colour bar for ease of reference. The top box is for $a = 0.001$ and the bottom box is for $a = 0.1$. All figures are plotted in the range $x \in [-20, 20]$, $y \in [-20, 20]$.

of Fr for subcritical flow, this demonstrates the promising potential of the fKP equation as a ‘sand-pit’ model to understand nonlinear Kelvin wave patterns.

In figure 4 we show a comparison between the fKP model and the fully nonlinear Euler system when $\sigma(\mathbf{x}) = a \exp(-\mathbf{x} \cdot \mathbf{x})$ for small amplitude forcings ($a = 0.001$, top box) and large amplitude forcings ($a = 0.1$, bottom box). The steady wave patterns, $\eta_f(\mathbf{x})$, for the fully nonlinear Euler system (top row in each box for the moving-pressure problem, middle

row in each box for the topographic-obstacle problem) show good agreement with the fKP steady wave patterns, $\eta(\mathbf{x})$, around $Fr = 1$. For smaller values of Fr , the fully nonlinear Euler system appears to exhibit no waves in both the moving-pressure and topographic-obstacle problems but we would expect these waves to be exponentially small in Fr and so hard to detect numerically (Pethiyagoda *et al.* 2021). We remark that the agreement between the different systems is also good for large amplitude forcings, at least around $Fr = 1$.

An important part of the analysis that we emphasise is the importance of the pole, κ_0 , in producing waves downstream; if we can eliminate this pole, then there is a possibility that a wave-free steady state could exist. Since κ_0 does not depend on the explicit form of the forcing term, $\sigma(\mathbf{x})$, then the forcing does not play a leading role in determining the wave pattern far downstream. It is often convenient to model $\sigma(\mathbf{x})$, taking the role as a ‘boat’, as a Dirac-delta function or Gaussian for a one-point wavemaker (see, for example, Whitham 1974; Katsis & Akylas 1987) or as a dipole modelling a two-point wavemaker (Noblesse *et al.* 2014; Miao & Liu 2015). We will now show that, through a judicious choice of $\sigma(\mathbf{x})$, corresponding to a localised forcing term, a nonlinear steady wave-free $\eta(\mathbf{x})$ exists.

4. Wave-free steady solutions

4.1. Weakly nonlinear model

Initially, we concentrate on the linear solution (3.5) to the weakly nonlinear fKP model, which in Cartesian coordinates ($\mathbf{k} = [k_x, k_y]^T$) can be written as

$$\eta_\ell(\mathbf{x}) = \frac{1}{2\pi} \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} \frac{3k_x^2 \hat{\sigma}_\ell(\mathbf{k})}{6(Fr - 1)k_x^2 + k_x^4 - 3k_y^2} \exp(i\mathbf{k} \cdot \mathbf{x}) dk_x dk_y. \quad (4.1)$$

As previously stated, the pole in the integrand is responsible for the far-field wave pattern as $x \rightarrow \infty$. Therefore, an obvious choice for $\hat{\sigma}_\ell(\mathbf{k})$ that eliminates this pole is

$$\hat{\sigma}_\ell(\mathbf{k}) = \frac{1}{3} \left[6(Fr - 1)k_x^2 + k_x^4 - 3k_y^2 \right] \hat{f}(\mathbf{k}), \quad (4.2)$$

where \hat{f} is the Fourier transform of an arbitrary function, $f(\mathbf{x})$, that is localised in physical space. In physical space, the linear solution is

$$\eta_\ell(\mathbf{x}) = \frac{1}{2\pi} \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} k_x^2 \hat{f}(\mathbf{k}) \exp(i\mathbf{k} \cdot \mathbf{x}) dk_x dk_y = -(f(\mathbf{x}))_{xx}. \quad (4.3)$$

A simple illustrative example is the choice $\hat{f}(\mathbf{k}) = -a\pi \exp(-(1/4)\mathbf{k} \cdot \mathbf{k})$, $a \in \mathbb{R}$. In this case, it is straightforward to see from (4.3) that

$$\eta_\ell(\mathbf{x}) = a (\exp(-\mathbf{x} \cdot \mathbf{x}))_{xx}, \quad (4.4)$$

which is, crucially, wave free in the far field; thus achieving our goal. Recall that η_ℓ and σ_ℓ satisfy (3.1) so, in order to satisfy the nonlinear problem (2.8), we simply insert (4.4) into the relation in (3.2).

As an example, when $a = 0.1$, $Fr = 1.0$, the nonlinear solutions η and σ in (3.2) are shown in figure 5 panels (a), (b) and (c), (d), respectively. This is our second main result of the paper; we can choose a localised forcing function that results in a nonlinear wave-free, localised free surface.

The forcing distribution in (3.2) is multi-signed, which is similar to the dipole model of a monohull used by Noblesse *et al.* (2014) and for the more arbitrary pressure shapes chosen in Miao & Liu (2015). Although it is not difficult to determine the solution in (3.2)

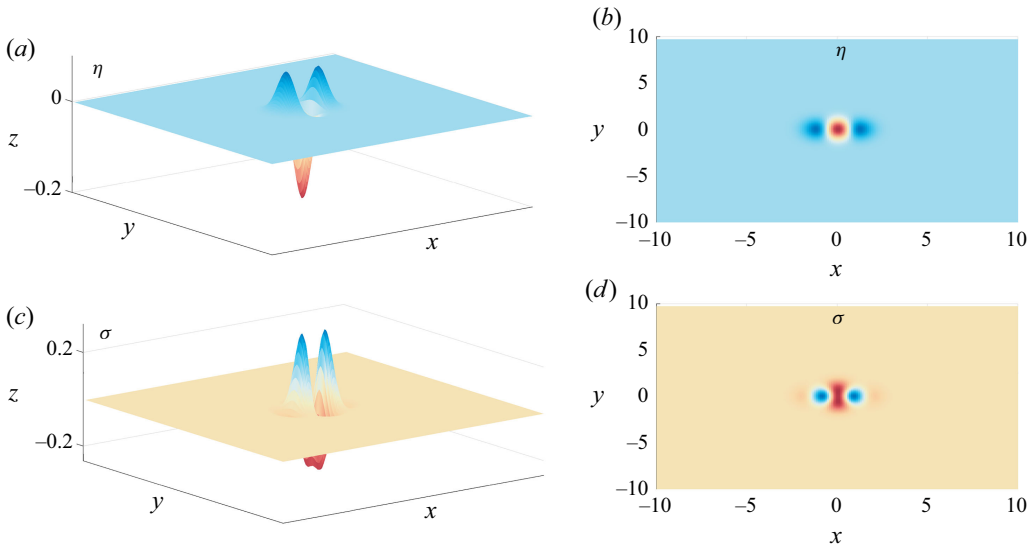


Figure 5. Steady weakly nonlinear wave-free solutions defined in (3.2). Panels (a–b) the nonlinear free surface, $\eta(\mathbf{x})$ for $a = 0.1$, $Fr = 1.0$. Panels (c–d) the forcing, $\sigma(\mathbf{x})$.

without resorting to a Fourier analysis, the strategy of eliminating the pole in the linear system does hint at a possible approach for minimising the waves in the Euler system, where the nonlinearity is not as straightforward as the quadratic term in (2.8), which is what we shall discuss in the next section.

4.2. Fully nonlinear system

We now extend the strategy of eliminating the pole of the linear system, as discussed in the previous section for the fKP equation, to the steady fully nonlinear Euler system described in (2.1)–(2.6). We perform a ‘Stokes-like’ perturbation expansion and write

$$\phi(\mathbf{x}, z) = x + \alpha\phi_\ell(\mathbf{x}, z) + \dots, \quad z_f(\mathbf{x}) = 1 + \alpha\eta_{f,\ell}(\mathbf{x}) + \dots, \quad \sigma(\mathbf{x}) = \alpha\sigma_\ell(\mathbf{x}) + \dots, \quad (4.5)$$

where $\alpha \ll 1$ and $\sigma(\mathbf{x})$ could represent $\sigma_{press}(\mathbf{x})$ or $\sigma_{topog}(\mathbf{x})$. We remind the reader that $z_f(\mathbf{x}) = 1 + \alpha\eta_{f,\ell}(\mathbf{x})$ and $z_f(\mathbf{x}) = 1 + \eta_f(\mathbf{x})$ now correspond to linear and nonlinear steady solutions of (2.1)–(2.6), respectively.

4.3. Moving-pressure problem

For the moving-pressure problem the linear solution for $\eta_\ell(\mathbf{x})$ in (4.5) can formally be written down using two-dimensional Fourier transforms

$$\eta_{f,\ell}(\mathbf{x}) = \frac{Fr^2}{2\pi} \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} \frac{\hat{\sigma}_\ell(\mathbf{k})|\mathbf{k}| \tanh |\mathbf{k}|}{Fr^2 k_x^2 - |\mathbf{k}| \tanh |\mathbf{k}|} \exp(i\mathbf{k} \cdot \mathbf{x}) d\mathbf{k}. \quad (4.6)$$

The denominator in (4.6) has poles that are responsible for the Kelvin wake pattern far downstream of the obstacle. Unlike the linear fKP analysis in §3, the poles cannot be written down in a simple-closed form. However, following the same strategy as the fKP analysis in §4, an obvious choice of $\hat{\sigma}_\ell(\mathbf{k})$ that eliminates the pole in (4.6) is

$$\hat{\sigma}_\ell(\mathbf{k}) = \frac{(ik_x)^n (ik_y)^m (Fr^2 k_x^2 - |\mathbf{k}| \tanh |\mathbf{k}|)}{Fr^2 |\mathbf{k}| \tanh |\mathbf{k}|} a \exp(-\mathbf{k} \cdot \mathbf{k}), \quad (4.7)$$

where $n, m \in \mathbb{N}$ and $a \in \mathbb{R}$. Imposing (4.7) results in a family of linear wave-free profiles

$$\eta_{f,\ell}(\mathbf{x}) = a (\exp(-\mathbf{x} \cdot \mathbf{x}))_{x^{(n)}y^{(m)}}, \quad (4.8)$$

where $x^{(n)}$ and $y^{(m)}$ represent the n th- x and m th- y derivatives, respectively. Here, we can choose $n = m = 0$ and in this sense the wave-free $\eta_{f,\ell}(\mathbf{x})$ in the fully nonlinear moving-pressure problem is less restricted than in the fKP system, where $\eta_{f,\ell}(\mathbf{x})$ has to be a second x -derivative of a localised two-dimensional function, see (4.3).

In contrast to the fKP model, where a nonlinear solution can be written down exactly (see (3.2)), we are unable to write down a simple exact nonlinear solution to the moving-pressure problem based on $\eta_{f,\ell}(\mathbf{x})$ and $\hat{\sigma}_\ell(\mathbf{k})$ in (4.8) and (4.7), respectively. We could go to higher-order α in (4.5) and choose $\hat{\sigma}(\mathbf{k})$ at each order judiciously to eliminate the higher-order poles, but the resulting expressions will become increasingly complicated, require inversion of a two-dimensional Fourier integral and it is not at all obvious that the corresponding series solution would be convergent. To avoid this, and inspired by the fact that the nonlinear free surface is identical to the linear free surface in the fKP analysis (see (3.2)), we directly determine $\sigma_{press}(\mathbf{x})$ by solving the nonlinear moving-pressure problem numerically as a nonlinear inverse problem for $\sigma_{press}(\mathbf{x})$ given a prescribed wave-free $\eta_f(\mathbf{x})$, as opposed to the so-called nonlinear forward problem of finding $\eta_f(\mathbf{x})$ given $\sigma_{press}(\mathbf{x})$.

We benchmark our results by choosing two different types of $\eta(\mathbf{x})$ from (4.8)

$$\eta_f(\mathbf{x}) = \eta_g(\mathbf{x}) = a \exp(-\mathbf{x} \cdot \mathbf{x}), \quad \eta_f(\mathbf{x}) = \eta_s(\mathbf{x}) = a (\exp(-\mathbf{x} \cdot \mathbf{x}))_{xx}. \quad (4.9)$$

The corresponding inversely found $\sigma_{press}(\mathbf{x})$ are shown in figure 6 for $\eta = \eta_g$ (top two rows) and $\eta = \eta_s$ (bottom two rows). We make the important observation that these are all localised in space, hence mimicking a finite ‘boat’. As can be seen from the top two rows, corresponding to $\sigma_{press}(\mathbf{x})$ when $\eta_g(\mathbf{x})$, for small Fr the forcing term has a single minimum which occurs at the origin, then as Fr increases to unity, two distinct minima emerge aligned to the x axis. Increasing Fr further results in the two minima being separated by a maximum. In the bottom row, corresponding to $\sigma_{press}(\mathbf{x})$ when $\eta_s(\mathbf{x})$, the previous description is reversed where instead of maxima there are minima and *vice versa*. Notice that for large Fr the shape of $\sigma_{press}(\mathbf{x})$ for $\eta_g(\mathbf{x})$ (top two rows) and $\eta_s(\mathbf{x})$ (bottom two rows) are similar, except for the opposite sign.

4.4. Topographic-obstacle problem

We now concentrate on the topographic-obstacle problem. Formally, the linear solution to (2.1)–(2.6) in this case is

$$\eta_{f,\ell}(\mathbf{x}) = \frac{Fr^2}{2\pi} \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} \frac{\hat{\sigma}_{topog}(\mathbf{k}) k_x^2 \operatorname{sech} |\mathbf{k}|}{Fr^2 k_x^2 - |\mathbf{k}| \tanh |\mathbf{k}|} \exp(i\mathbf{k} \cdot \mathbf{x}) dk_x dk_y. \quad (4.10)$$

An important remark is that the denominator of the integrand in (4.10) is identical to that for the moving-pressure distribution problem in (4.6) and hence the location and strength of the poles in the complex plane are also identical. However, unlike the moving-pressure problem, we would not expect to be able to solve the resulting numerical inverse problem of prescribing the shape of the $z_f(\mathbf{x})$ and then solving (2.1)–(2.6) to find $\sigma_{topog}(\mathbf{x})$, as it is likely to be ill posed as it is in the two-dimensional problem (Robbins *et al.* 2023) and sophisticated optimisation techniques, beyond the present scope of this paper, would be expected to be required. We emphasise that, for the moving-pressure problem, there is no numerical evidence that the inverse problem is ill posed.

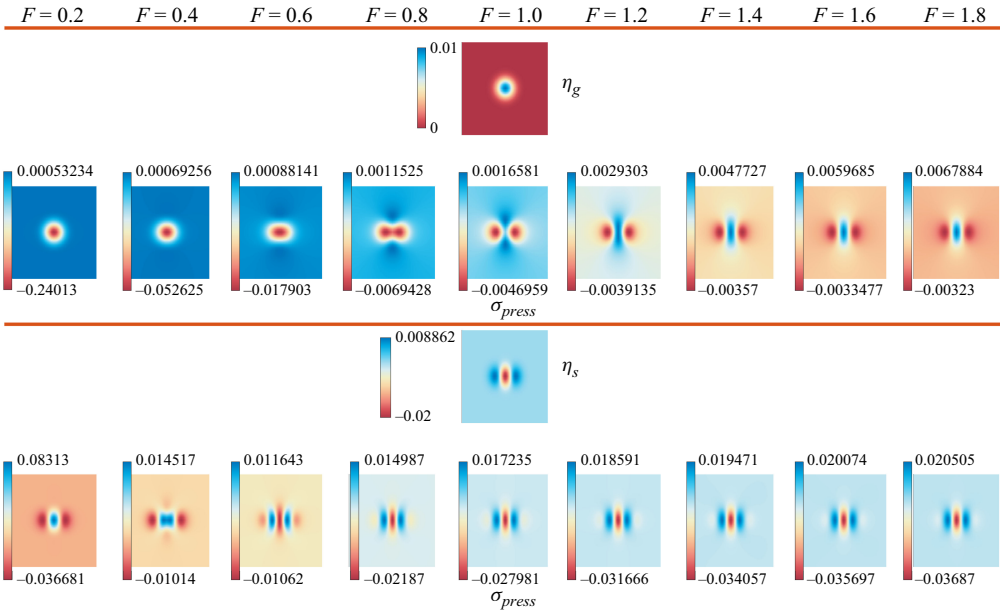


Figure 6. Fully nonlinear moving-pressure problem as Fr is varied. Each panel is plotted in the range $x \in [-10, 10]$, $y \in [-10, 10]$. First and second rows: wave-free profile $\eta_g(\mathbf{x})$ and corresponding $\sigma_{press}(\mathbf{x})$, respectively. Third and fourth rows: wave-free profile $\eta_s(\mathbf{x})$ and corresponding $\sigma_{press}(\mathbf{x})$, respectively. In both cases $a = 0.01$ (see (4.9)).

However, due to the identical denominator, we can still make progress as the same $\sigma_\ell(\mathbf{k})$ that eliminates the linear wave pattern in the moving-pressure problem will also eliminate the linear wave pattern in the topographic-obstacle problem. It is not unreasonable to expect, therefore, that choosing $\sigma_{topog}(\mathbf{x}) = \sigma_{press}(\mathbf{x})$ will result in a significant reduction in the nonlinear wave signature in the forward topographic-obstacle problem. The strategy is as follows. First, we prescribe $\eta_f(\mathbf{x})$ for the moving-pressure problem and solve the nonlinear inverse problem for $\sigma_{press}(\mathbf{x})$. Next, we set $\sigma_{topog}(\mathbf{x}) = \sigma_{press}(\mathbf{x})$ and solve the nonlinear forward topographic-obstacle problem for $\eta_f(\mathbf{x})$.

This process is shown in figure 7 for the specific case of $\eta_f(\mathbf{x}) = \eta_g(\mathbf{x})$ in (4.9). As can be seen in this middle left panel, the wave signature of the nonlinear $\eta_f(\mathbf{x})$ for the topographic-obstacle problem is present but barely visible. As a trial case to highlight this minimisation, on the bottom row, we also show $\eta_f(\mathbf{x})$ (bottom-left panel) as a solution to a forwards problem with $\sigma_{topog}(\mathbf{x}) = \hat{a} \exp(-\mathbf{x} \cdot \mathbf{x})$ where \hat{a} is chosen so that the integral of $\sigma_{topog}(\mathbf{x})$ over the domain is identical to the integral of the topography in the middle-right panel (value in caption). The comparison between the two resulting $\eta_f(\mathbf{x})$ in the middle- and bottom-left panels is striking. These results are for $Fr = 1.2$ but similar results exist for $Fr \in [0.2, 1.8]$ that we sampled. We leave the highly non-trivial problem of inversely finding a localised topographic obstacle given a prescribed free surface as an open challenge.

4.5. Wave resistance

It is illuminating to briefly discuss the physical implications of wave-free solutions. To this end, and referring to figure 1, we apply the momentum integral equation for inviscid flow (e.g. Batchelor 1967) over a control volume confined between the free surface Γ_2 the bottom Γ_0 and the four vertical sides Γ_1 , Γ_3 , Γ_4 and Γ_5 that are parallel to the x and y

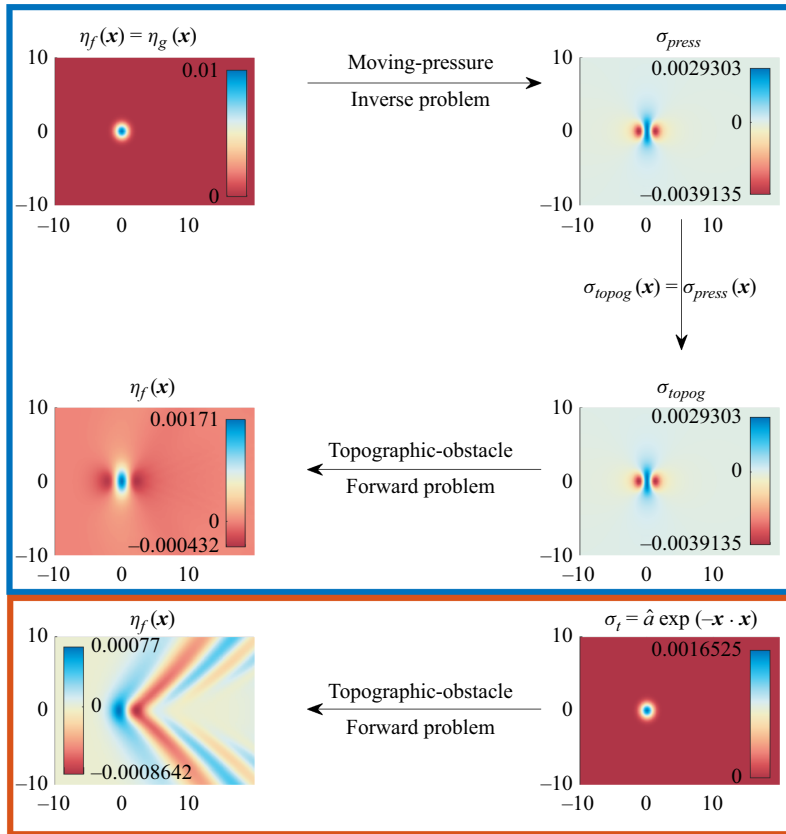


Figure 7. Fully nonlinear Euler system when $Fr = 1.2$. Blue box: top left: $\eta_f(\mathbf{x})$ for the moving-pressure distribution given by (4.9). Top right: the pressure-distribution, $\sigma_{press}(\mathbf{x})$ that results in (4.9) when $a = 0.01$. Middle right: the prescribed topographic-obstacle $\sigma_{topog}(\mathbf{x}) = \sigma_{press}(\mathbf{x})$. Middle-left: the resulting free surface for $\sigma_{topog}(\mathbf{x})$. Red box: bottom right: Gaussian topographic obstacle with $\hat{a} = 0.001653$ chosen so it has identical mass to $\sigma_{topog}(\mathbf{x})$. Bottom left: free-surface response for the Gaussian topography.

axes. Taking the component in the horizontal direction, we obtain

$$\mathcal{F} = \iint_{\Gamma_1} (p + \rho u^2) dS - \iint_{\Gamma_3} (p + \rho u^2) dS + \iint_{\Gamma_4} v(\rho u) dS + \iint_{\Gamma_5} (-v)(\rho u) dS, \quad (4.11)$$

where $(u, v) = (\phi_x, \phi_y)$. The first two terms on the right-hand side represent the flux of total horizontal momentum (e.g. Lighthill 1978) through the downstream and upstream faces, Γ_1 and Γ_3 , respectively, and the second two terms on the right-hand side represent the flux of horizontal momentum out of the lateral faces Γ_4 and Γ_5 . On the left-hand side

$$\mathcal{F} = \mathbf{i} \cdot \iint_{\Gamma_2} p \nabla \eta dS \quad \text{or} \quad -\mathbf{i} \cdot \iint_{\Gamma_0} p \nabla \sigma dS, \quad (4.12)$$

represents either the external pressure force acting on the free surface or the force imposed by the topography on the fluid (or a combination of both). In the former case \mathcal{F} coincides with the wave resistance defined by Havelock (1917). We interpret (4.11) as meaning that the total external force exerted on the fluid, either by the surface pressure or by the topography, is equal to the net transport of horizontal momentum out through the side faces. In the case of our special topography which localises the surface disturbance, if

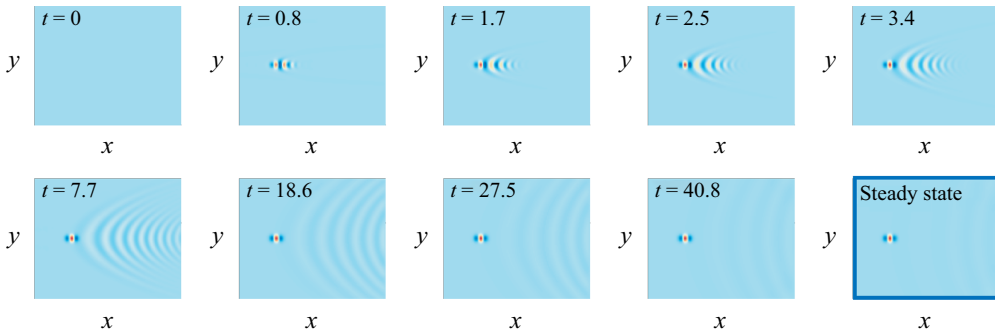


Figure 8. Nonlinear KP time-dependent solution, $\eta(\mathbf{x}, t)$, of (2.8), $x \in [-10, 30]$, $y \in [-20, 20]$, starting from (5.1) with forcing in (4.4), $Fr = 1.0$, $a = 0.1$. The corresponding animation is shown in movie_3.mp4 in the supplementary material.

the control volume is sufficiently large the right-hand side of (4.11) vanishes so that no net momentum is imparted to the surrounding fluid. Then $\mathcal{F} = 0$ and the wave resistance is zero, meaning that a ‘boat’ represented by the moving pressure experiences zero wave drag, or the topography exerts a vanishing net force upon the fluid.

5. Stability of nonlinear wave-free steady solutions

We now examine the stability of the wave-free steady solutions in the previous section by solving a suite of IVPs. First, we investigate the time-dependent fKP in (2.8) and then the time-dependent fully nonlinear Euler system in (2.1)–(2.6).

5.1. The fKP model equation

We choose the initial condition

$$\eta(\mathbf{x}, t = 0) = 0, \quad (5.1)$$

which represents a flat free surface. To benchmark our results, initially, we solve the IVP with a Gaussian forcing function, $\sigma(\mathbf{x}) = a \exp(-\mathbf{x} \cdot \mathbf{x})$ and a dipole forcing $\sigma(\mathbf{x}) = a [\exp(-\mathbf{x} \cdot \mathbf{x})]_{xx}$ ($a = 0.001$) and confirm that the weakly nonlinear system evolves towards a steady weakly nonlinear Kelvin wake, see movie_1.mp4 and movie_2.mp4 in the supplementary material.

Now, we concentrate on our special form of forcing in (4.4) for the weakly nonlinear fKP model and, to be consistent with the steady results in figure 5, choose $Fr = 1.0$, $a = 0.1$. Figure 8 (corresponding to the animation movie_3.mp4 in the supplementary material) shows the resulting time-dependent behaviour starting from (5.1). As can be seen from the time snapshots, for approximately $0 < t < 7.7$ (top row) a curved wave pattern emerges directly downstream of the forcing. Then, for approximately $t \geq 7.7$ (bottom row) this wake pattern propagates downstream in the form of curved ‘ripples’ before eventually separating and leaving the steady state, $\eta(\mathbf{x}, t)$, which we have confirmed is the steady state in (3.2). This behaviour is generic for a wide range of parameter space ($Fr \in [0.2, 1.8]$ and $a \in [-0.1, 0.1]$) and has been thoroughly checked for numerical convergence. Finally, as well as the initial condition in (5.1), numerical simulations provide evidence that the steady state is also stable to two- and three-dimensional perturbations, as shown in movie_4.mp4 and movie_5.mp4. This is strong numerical evidence that the wave-free steady state is not only stable but is also the asymptotic state of the system as $t \rightarrow \infty$. Furthermore, the waves emitted downstream in the initial stage of evolution appear different to the classical

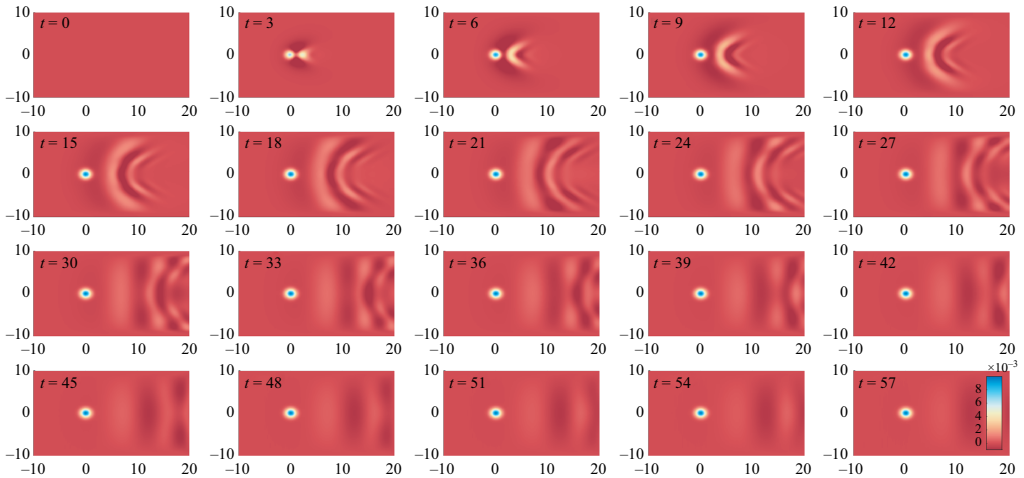


Figure 9. Fully nonlinear moving-pressure time-dependent solution for $\eta_f(\mathbf{x}, t)$, $x \in [-10, 30]$, $y \in [-10, 10]$, with $\sigma_{press}(\mathbf{x})$ given in (4.7) for $\eta_g(\mathbf{x})$ in (4.9), $Fr = 1.0$, $a = 0.01$.

Kelvin wake pattern and are similar in nature to the capillary-gravity waves, as shown in Chepelianskii, Chevy & Raphael (2008), Closa, Chepelianskii & Raphael (2010), except these ripple waves only occur downstream. These ripples may be related to the parabolic ‘ripples’ solutions of the unforced KP equation, as reported in Johnson & Thompson (1978), Nakamura (1981), Zhang *et al.* (2023, 2024) and initial investigations indicate that the ripples we report are indeed parabolic; a detailed analysis of these forced ripple solutions is beyond the scope of this paper and is currently being written up in a separate research paper.

5.2. Fully nonlinear Euler system

We now investigate the stability of the fully nonlinear Euler steady states for the moving-pressure and topographic-obstacle problems. Concentrating initially on the moving-pressure problem, first, we impose a free surface in (4.9) to obtain the pressure distribution, $\sigma_{press}(\mathbf{x})$ before setting a flat free surface with a uniform flow as an initial condition

$$\eta_f(\mathbf{x}, t = 0) = 0, \quad \phi(\mathbf{x}, z, t = 0) = x. \quad (5.2)$$

We emphasise that σ_{press} is present at $t = 0$. In figure 9 we show that $\eta_f(\mathbf{x}, t)$ evolves towards the steady wave-free free surface in the moving-pressure problem with forcing in (4.7) for the steady free-surface $\eta_g(\mathbf{x})$ in (4.9). The ripples that get emitted downstream are similar in form to that in the fKP equation but we would expect them to be circular, instead of parabolic, as there is no weak y dependence in the fully nonlinear Euler system.

For the topographic-obstacle problem we first perform the process described in figure 7 to find $\sigma_{topog}(\mathbf{x}) = \sigma_{press}(\mathbf{x})$ using a free surface in (4.9). We then deform the fluid domain using this topographic forcing, set $\eta_f(\mathbf{x}) = 0$ and solve Laplace’s equation in the fluid domain together with (2.2)–(2.6) to obtain an initial condition for $\phi(\mathbf{x}, z)$. Once time is advanced, the subsequent simulations show that the system tends towards the ‘almost’ wave-free steady state, as shown in figure 10.

Viewed altogether, these calculations show the remarkable stability of the wave-free steady state in both the weakly nonlinear fKP model and fully nonlinear Euler system; our third, and perhaps most significant result of the paper. A carefully constructed forcing

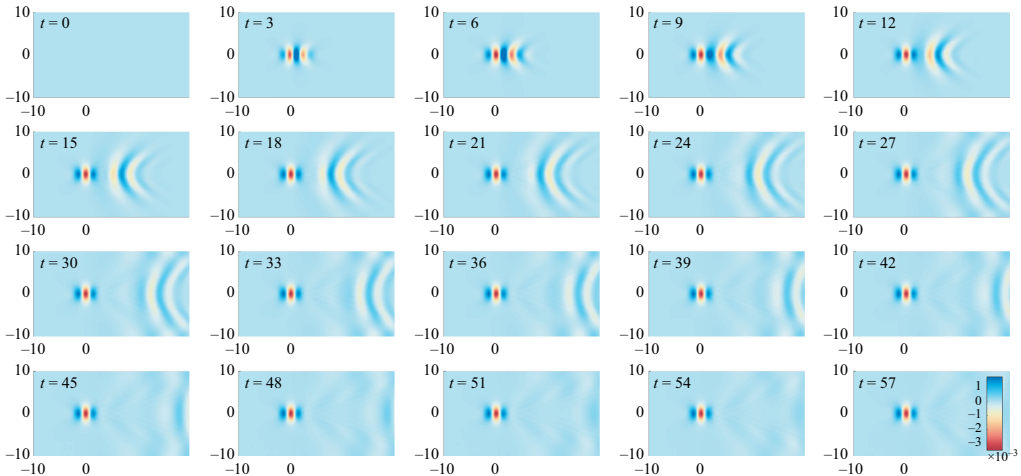


Figure 10. Fully nonlinear topographic-obstacle time-dependent solution for $\eta_f(\mathbf{x}, t)$, $x \in [-10, 30]$, $y \in [-10, 10]$, with $\sigma_{topog}(\mathbf{x})$ given in (4.7) for $\eta_s(\mathbf{x})$ in (4.9), $Fr = 1.0$, $a = 0.01$.

term, as described in the previous section, will result in a wave-free profile that can be observed in a physical experiment.

6. Conclusions and perspective

We have demonstrated that the fKP equation is capable of producing the v-shaped Kelvin wave pattern that is observed in practice when an object moves along the surface of water, and we have characterised the associated wedge angle as a function of the Froude number. More importantly, we have shown that, for the fKP and the fully nonlinear Euler system, a judiciously chosen pressure forcing can produce a steady wave-free surface, meaning that the disturbance caused by the forcing is confined to its neighbourhood so that the surface is flat in the far field. In addition, if we use this pressure forcing as a proxy for a topographic obstacle, then the resulting far-field waves in the free-surface profile are significantly diminished. Crucially, using numerical simulations we have demonstrated that the wave-free states are stable in the sense that they are reached as $t \rightarrow \infty$ for a suitably posed IVP.

Despite the simplicity of our mathematical argument, this appears to be the first time that wave-free steady solutions have been constructed for the fKP system and their stability properties calculated. Our results provide some evidence that it may be possible to eliminate the Kelvin wake in a real-world setting. However, further work is needed to establish whether wave-free solutions can be observed in a laboratory experiment. On this point, we highlight the work of Chen & Sharma (1995) and Chen, Sharma & Stuntz (2003) where, using a weakly nonlinear model, similar to the fKP they were able to design a zero wave resistance moving body in a narrow confined channel and these were compared with experiments. Another interesting recent work to highlight is by Euve *et al.* (2024) in which asymmetric wake patterns have been observed experimentally and whose experimental approach may be relevant for the wave-free solutions in this paper. In addition, it would also be interesting to examine the transverse stability of the two-dimensional solutions discovered in Keeler *et al.* (2017, 2021) at $Fr = 1$, using the computational framework developed here.

We emphasise that we have identified wave-free steady states over a large range of the Froude number and forcing amplitude. Regarding wake elimination in real-world applications, our results are at this point only suggestive. Nevertheless, this may ultimately pave the way to improving marine vessel design, including for example the development of ‘stealth’ boats, by reducing the wave drag and minimising the wake signature. It would be interesting to try to perform an experiment using one of our pressure forcings in a flow tuned to the correct Froude number to try to realise our prediction in the laboratory. Our results for topographic obstacles suggest the possibility of designing underwater structures that minimise or eliminate any disruption to the water surface.

For both the fKP model equation and fully nonlinear Euler system studied here, the free-surface profile is extremely sensitive to small changes in the forcing function, as is demonstrated when we use the pressure distribution as a proxy for a topographic obstacle and a downstream Kelvin wake is observed, albeit small of amplitude. Whether a special topographic forcing exists that results in a wave-free surface disturbance in the fully nonlinear Euler system is an open question. The determination of this special topographic forcing will likely require bespoke computational methods to solve the nonlinear inverse problem that arises and we leave this for future work. Finally, it would also be interesting to compare our results with other works that have attempted to minimise the wave drag, including using a ‘pressure-patch model (Tuck & Lazauskas 2001) and optimisation methods on an actual boat hull (Dambrine & Pierre 2020).

Supplementary movies. Supplementary movies are available at <https://doi.org/10.1017/jfm.2025.10225>.

Funding. J. S. K acknowledges funding from the Leverhulme Trust; ECF-2021-017.

Declaration of interests. The authors report no conflict of interest.

Data availability statement. The data are available upon request.

Appendix A. Numerical method

The fKP model and the fully nonlinear Euler system are solved in the weak form (as discussed below for each individual system). The common features of the discretisation that apply to both systems are that (i) the equations are discretised using finite elements and the open-source oomph-lib package (Heil & Hazel 2006), (ii) the test functions, ψ , are piecewise cubic functions and the spatial coordinates and unknown field variables are interpolated using ψ , (iii) steady states are found using Newton iterations and (iv) time stepping is performed using a backwards Euler order-two method.

A.1 Weak form of the fKP model

To proceed, (2.8) can be written in conservative form

$$\nabla \cdot \mathbf{U} = 0, \quad \mathbf{U} = \left[\eta_t + (Fr - 1)\eta_x - \frac{3}{2}\eta\eta_x - \frac{1}{6}\eta_{xxx} - \frac{1}{2}\sigma_x, -\frac{1}{2}\eta_y \right]^T. \quad (\text{A1})$$

We multiply (A1) by a test function, ψ , multiply over the domain and then use the divergence theorem to obtain the weak residual equation

$$\mathcal{R}_{fKP}(\eta) \equiv \iint_{\Omega} \psi (\nabla \cdot \mathbf{U}) \, dV \equiv \iint_{\partial\Omega} \psi (\mathbf{U} \cdot \mathbf{n}) \, dS - \iint_{\Omega} \nabla \psi \cdot \mathbf{U} \, dV = 0. \quad (\text{A2})$$

To discretise and then solve (A2), we truncate the infinite mathematical domain in the range $x \in [-L_x, L_x]$, $y \in [-L_y, L_y]$ and typically we choose $L_x = L_y = 30$, choose 110 horizontal and vertical elements and choose a time step of $\Delta t = 0.1$. On $y = \pm L_y$, we set $\mathbf{U} \cdot \mathbf{n} = \eta_y = 0$ and on $x = -L_x$ we impose $\eta = \eta_x = 0$. On $x = L_x$ we have to satisfy an

unknown radiation condition. For simplicity, and to be consistent with Katsis & Akylas (1987) we set $\eta = 0$ and $\mathbf{U} \cdot \mathbf{n} = 0$ on this boundary.

A.2 Weak form of the fully nonlinear Euler system

A full description of this numerical formulation can be found in Keeler & Blyth (2024), where a lot of the details can be carried over to the three-dimensional case we consider here. Briefly summarising, we truncate the infinite mathematical domain in the range $x \in [-L_x, L_x]$, $y \in [-L_y, L_y]$ and typically we choose $L_x = L_y = 20$. Far upstream and downstream we introduce ‘sponge layers’ so that waves are forced to become locally flat. To obtain the weak formulation, we multiply (2.1) by a test function, $\psi(\mathbf{x})$, integrate over the computational domain, $\Omega(t)$, and use the divergence theorem to get our bulk residual

$$\mathcal{R}_{Bulk}(\mathbf{x}, \phi(\mathbf{x})) \equiv \iint_{\partial\Omega(t)} (\mathbf{n} \cdot \nabla \phi) \psi \, dS - \iiint_{\Omega(t)} \nabla \phi \cdot \nabla \psi \, dV = 0, \quad (\text{A3})$$

where $\partial\Omega(t)$ represents the boundary of $\Omega(t)$, \mathbf{n} is the outwards-pointing unit normal vector on each part of the boundary and dV , dS are differential volume and area elements, respectively. We decompose the boundary into 6 parts; $\partial\Omega(t) = \Gamma_0 + \Gamma_1 + \Gamma_2(t) + \Gamma_3 + \Gamma_4 + \Gamma_5$, see figure 1. The kinematic boundary condition on Γ_2 (2.2), uniform-flow condition on Γ_1 (2.5) and no-penetration condition (2.6), can be incorporated directly into (A3). As there is a ‘sponge layer’ near the boundaries at Γ_4 and Γ_5 we impose $\mathbf{n} \cdot \nabla \phi = 0$ on these lateral boundaries which can also be incorporated as a natural boundary condition in (A3). Finally, we impose the uniform-flow condition on Γ_3 as an essential boundary condition by setting $\phi = -L$ on Γ_3 .

To satisfy (2.3) we multiply by a test function and integrate over the free surface to obtain

$$\mathcal{R}_{Dyn}(\mathbf{x}, \phi(\mathbf{x})) \equiv \iint_{z=z_f(\mathbf{x},t)} \left(\phi_t + \frac{1}{2} |\nabla \phi|^2 + \frac{1}{Fr^2} z_f + \sigma_{press} - \frac{1}{2} - \frac{1}{Fr^2} \right) \psi \, dS = 0. \quad (\text{A4})$$

For the forward problem, the steady forms of (A3) and (A4) are solved to determine the free-surface location, $z_f(\mathbf{x}, t)$, and the velocity potential in the fluid, $\phi(\mathbf{x}, t)$ with a prescribed forcing whilst for the inverse problem we solve for $\sigma_{press}(\mathbf{x})$ given a prescribed $z_f(\mathbf{x})$.

We typically chose $70 \times 40 \times 2$ elements in the $x \times y \times z$ directions and chose a time step $\Delta t = 1.0$. To account for the unknown free surface, $\eta(\mathbf{x}, t)$, we use a structured brick mesh with a spine-node update strategy. An important remark is that despite calculating quantities in the bulk fluid as well as the free surface, the corresponding Jacobian matrix is sparse, in contrast to the Green’s function approaches. We have not used any Newton–Krylov methods, nor applied pre-conditioning, the use of which would result in significant computational speed-up and we leave this as an open challenge.

REFERENCES

- BATCHELOR, G.K. 1967 *An Introduction to Fluid Dynamics*. Cambridge University Press.
- BISHOP, M.J. 2004 A posteriori evaluation of strategies of management: the effectiveness of no-wash zones in minimizing the impacts of boat-wash on macrobenthic infauna. *Environ. Manage.* **34** (1), 140–149.
- BUTTLE, N.R., PETHIYAGODA, R., MORONEY, T.J. & MCCUE, S.W. 2018 Three-dimensional free-surface flow over arbitrary bottom topography. *J. Fluid Mech.* **846**, 166–189.
- CHEN, X.-N. & SHARMA, S.D. 1995 A slender ship moving at a near-critical speed in a shallow channel. *J. Fluid Mech.* **291**, 263–285.

- CHEN, X.-N., SHARMA, S.D. & STUNTZ, N. 2003 Zero wave resistance for ships moving in shallow channels at supercritical speeds. Part 2. Improved theory and model experiment. *J. Fluid Mech.* **478**, 11–124.
- CHEPELIANSKII, A.D., CHEVY, F. & RAPHAEL, E. 2008 Capillary-gravity waves generated by a slow moving object. *Phys. Rev. Lett.* **100** (7), 074504.
- CLOSA, F., CHEPELIANSKII, A.D. & RAPHAEL, E. 2010 Capillary-gravity waves generated by a sudden object motion. *Phys. Fluids.* **22** (5), 052107.
- DAMBRINE, J. & PIERRE, M. 2020 Regularity of optimal ship forms based on Michell’s wave resistance. *Appl. Maths Optim.* **82** (1), 23–62.
- DARMON, A., BENZAQUEN, M. & RAPHAËL, E. 2014 Kelvin wake pattern at large Froude numbers. *J. Fluid Mech.* **738**, R3.
- EUVE, L.-P., MAUREL, A., PETITJEANS, P. & PAGNEUX, V. 2024 Asymmetrical wakes over anisotropic bathymetries. *J. Fluid. Mech.* **984**, R6.
- GROSENBAUGH, M.A. & YEUNG, R.W. 1989 Nonlinear bow flows—an experimental and theoretical investigation. In *Seventeenth Symposium on Naval Hydrodynamics*, pp. 195–213.
- HAVELOCK, T.H. 1908*a* The effect of shallow water on wave resistance. *Proc. R. Soc. Lond. A* **100**, 499–505.
- HAVELOCK, T.H. 1908*b* The propagation of groups of waves in dispersive media, with application to waves on water produced by a travelling disturbance. *Proc. R. Soc. Lond. A* **80**, 398–430.
- HAVELOCK, T.H. 1917 Some cases of wave motion due to a submerged obstacle. *Proc. R. Soc. Lond. A* **93** (654), 520–532.
- HEIL, M. & HAZEL, A.L. 2006 Oomph-lib—an object-oriented multi-physics finite-element library. In *Fluid-Structure Interaction*, pp. 19–49. Springer.
- JOHNSON, R.S. & THOMPSON, S. 1978 A solution of the inverse scattering problem for the Kadomtsev–Petviashvili equation by the method of separation of variables. *Phys. Lett. A* **66** (4), 279–281.
- KATAOKA, T. & AKYLAS, T.R. 2023 Nonlinear Kelvin wakes and exponential asymptotics. *Physica D* **454**, 133848.
- KATSIS, C. & AKYLAS, T.R. 1987 On the excitation of long nonlinear water waves by a moving pressure distribution. part 2. three-dimensional effects. *J. Fluid Mech.* **177**, 49–65.
- KEELER, J.S. 2014 Free-surface flow over bottom topography. PhD thesis, Universty of East Anglia and University of Adelaide, Australia.
- KEELER, J.S., BINDER, B.J. & BLYTH, M.G. 2017 On the critical free-surface flow over localised topography. *J. Fluid Mech.* **832**, 73–96.
- KEELER, J.S. & BLYTH, M.G. 2024 On the stability of fully nonlinear hydraulic-fall solutions to the forced water-wave problem. *J. Fluid Mech.* **993**, A9.
- KEELER, J.S., BLYTH, M.G. & KING, J.R. 2021 Termination points and homoclinic glueing for a class of inhomogeneous nonlinear ordinary differential equations. *Nonlinearity* **34** (1), 532–561.
- KELVIN, L. 1887 On ship waves. *Proc. Inst. Mech. Engrs* **38** (1), 409–434.
- LANNES, D. 2013 *The Water Waves Problem*. American Mathematical Society.
- LI, Y. & ELLINGSEN, S. 2016 Ship waves on uniform shear current at finite depth: wave resistance and critical velocity. *J. Fluid Mech.* **791**, 539–567.
- LIGHTHILL, M.J. 1978 *Waves in Fluids*. Cambridge University Press.
- LUSTRI, C.J. & CHAPMAN, S.J. 2013 Steady gravity waves due to a submerged source. *J. Fluid Mech.* **732**, 660–686.
- MIAO, S. & LIU, Y. 2015 Wave pattern in the wake of an arbitrary moving surface pressure disturbance. *Phys. Fluids.* **27** (12), 122102.
- NAKAMURA, A. 1981 Decay mode solution of the two-dimensional KdV equation and the generalized Bäcklund transformation. *J. Math. Phys.* **22** (11), 2456–2462.
- NOBLESSE, F., HE, J., ZHU, Y., HONG, L., ZHANG, C., ZHU, R. & YANG, C. 2014 Why can ship wakes appear narrower than Kelvin’s angle? *Euro. J. Mech.-B* **46**, 164–171.
- PEDERSEN, G. 1988 Three-dimensional wave patterns generated by moving disturbances at transcritical speeds. *J. Fluid Mech.* **196**, 39–63.
- PETHIYAGODA, R., MCCUE, S.W. & MORONEY, T.J. 2014 What is the apparent angle of a Kelvin ship wave pattern? *J. Fluid Mech.* **758**, 468–485.
- PETHIYAGODA, R., MCCUE, S.W. & MORONEY, T.J. 2015 Wake angle for surface gravity waves on a finite depth fluid. *Phys. Fluids.* **27** (6), 061701.
- PETHIYAGODA, R., MORONEY, T.J., LUSTRI, C.J. & MCCUE, S.W. 2021 Kelvin wake pattern at small Froude numbers. *J. Fluid Mech.* **915**, A126.
- PETHIYAGODA, R., MORONEY, T.J., MACFARLANE, G.J., BINNS, J.R. & MCCUE, S.W. 2018 Time-frequency analysis of ship wave patterns in shallow water: modelling and experiments. *Ocean Engng* **158**, 123–131.

- RABAUD, M. & MOISY, F. 2013 Ship wakes: Kelvin or mach angle? *Phys. Rev. Lett.* **110** (21), 214503.
- ROBBINS, C., BLYTH, M.G., MACLEAN, J. & BINDER, B.J. 2023 A method to calculate inverse solutions for steady open channel free-surface flow. *J. Fluid Mech.* **977**, A46.
- TRINH, P.H. & CHAPMAN, S.J. 2014 The wake of a two-dimensional ship in the low-speed limit: results for multi-cornered hulls. *J. Fluid Mech.* **741**, 492–513.
- TRINH, P.H., CHAPMAN, S.J. & VANDEN-BROECK, J.-M. 2011 Do waveless ships exist? results for single-cornered hulls. *J. Fluid Mech.* **685**, 413–439.
- TUCK, E.O. & LAZAUSKAS, L. 2001 Free-surface pressure distributions with minimum wave resistance. *ANZIAM J.* **43**, E75–E101.
- WHITHAM, G.B. 1974 *Linear and Nonlinear Waves*. John Wiley.
- ZHANG, Z., GUO, Q. & STEPANYANTS, Y. 2024 Highly localized horseshoe ripplons and solitons in positive dispersion media. *Wave Motion* **128**, 103326.
- ZHANG, Z., YANG, X., GUO, Q. & CAO, Y. 2023 Rare decaying ripple solutions within the KP equation. *Physica D* **456**, 133920.
- ZHU, Y., HE, J., ZHANG, C., WU, H., WAN, D., ZHU, R. & NOBLESSE, F. 2015 Farfield waves created by a monohull ship in shallow water. *Euro. J. Mech.-B* **49**, 226–234.
- ZHU, Y., MA, C., WU, H., HE, J., ZHANG, C., LI, W. & NOBLESSE, F. 2016 Farfield waves created by a catamaran in shallow water. *Euro. J. Mech.-B* **59**, 197–204.
- ZHU, Y., WU, H., HE, J., ZHANG, C., LI, W. & NOBLESSE, F. 2018 Hogner model of wave interferences for farfield ship waves in shallow water. *Appl. Ocean Res.* **73**, 127–140.